

# Design criteria for wind-loaded structures beyond Eurocode

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with contributions from  
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## Challenges

When Eurocode is not (directly) applicable: How to find equivalent design criteria?

- Illustration 1: Equivalent design loads for retractable structures
- Illustration 2: Translating results from advanced numerical models into design values

How to open up the path to more optimal (economical) designs with advanced analysis methods?

- How to deal with hidden safety in Eurocode?



## Background: Eurocode design criteria



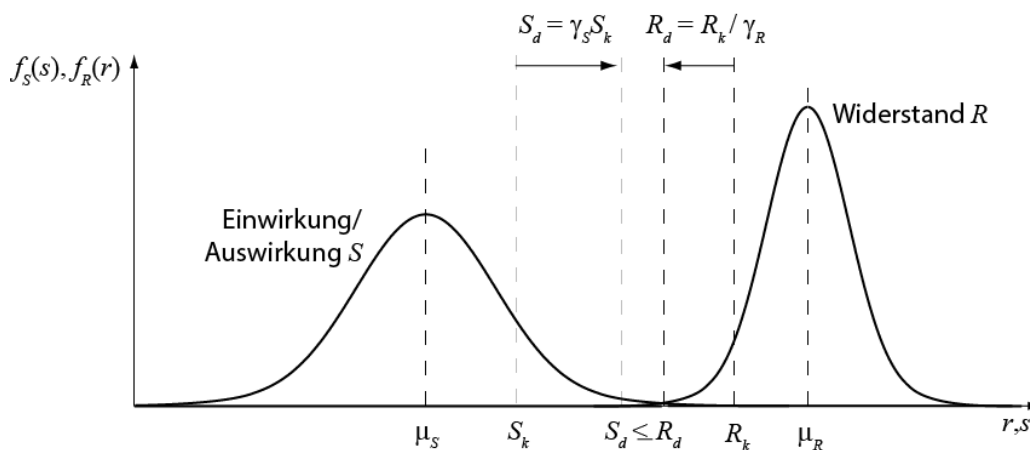
## Design of structures following Eurocode

Eurocode 0 (EN 1990) prescribes basis of design:

- Partial safety factor concept (LRFD)
- Balance between safe and economic
- Developed for and applicable to standard design situations



# Background to partial safety factor design (Eurocode)



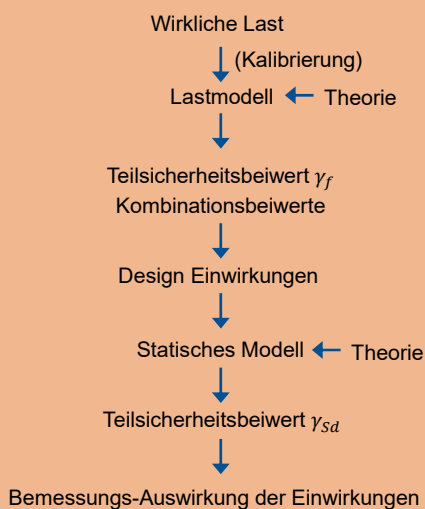
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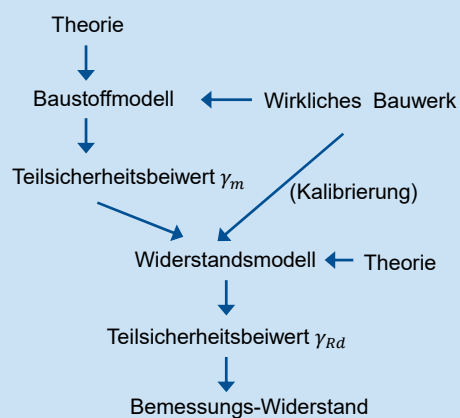
# Nachweisverfahren nach Eurocode



## Einwirkungsseite



## Widerstandseite



!  $\leq$

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## Design of a retractable roof



## Retractable roof



- Roof area: 30'000m<sup>2</sup>
- Under strong winds, the roof is automatically retracted
- Based on wind measurements
- Availability of the retraction process in case of strong winds: 99%

## Concept for design criteria

- Reliability of the roof must correspond to the reliability implied by Eurocode design
- During a strong wind event, the roof is not retracted with probability 1%

It follows:

- $P_F$  of the roof in its deployed state under strong winds  $\leq \frac{p_F}{0.01} = 100 p_F$  of a code-based design
- Probabilistic model must enable a relative comparison of reliability

## Design criteria

- Simplest reference model: Failure at  $S \geq R$

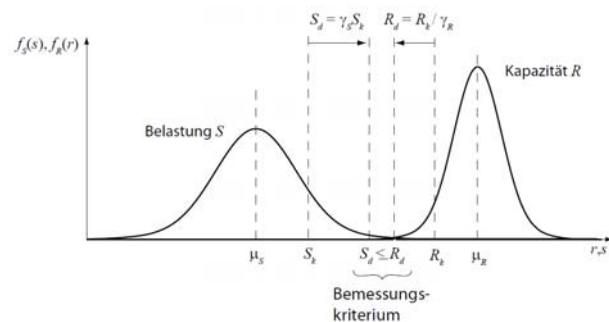
- Load effects:

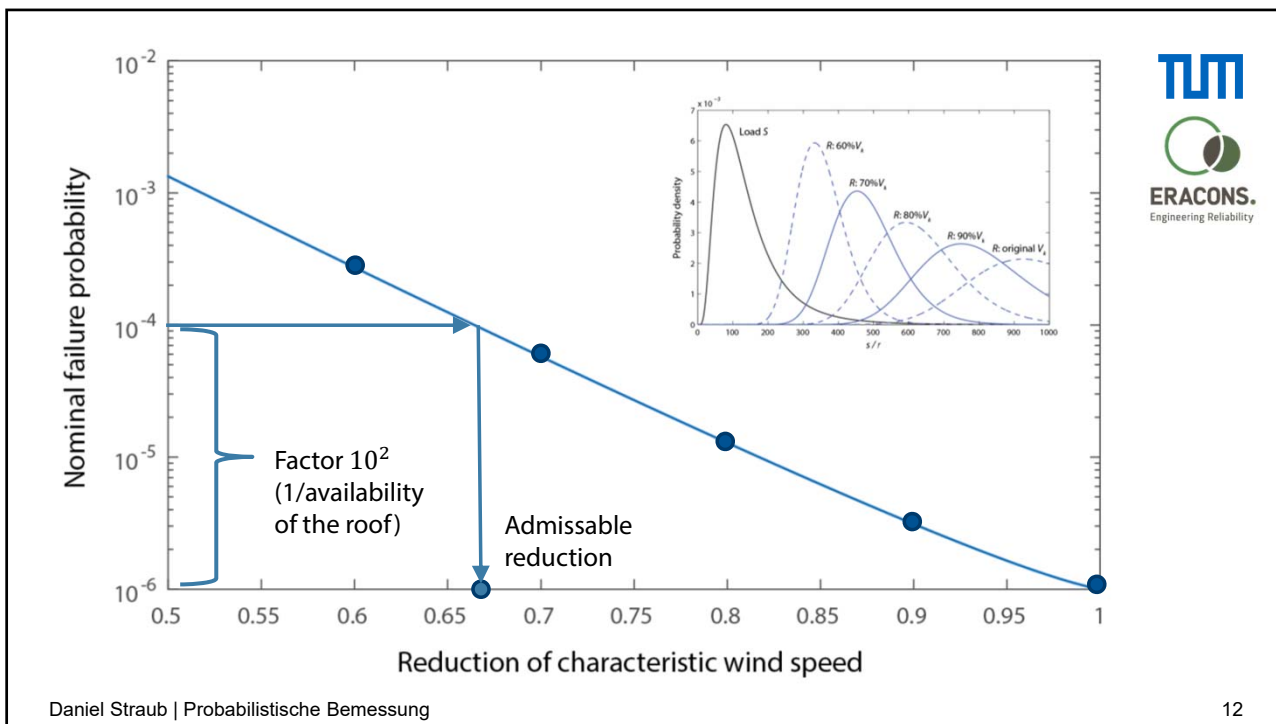
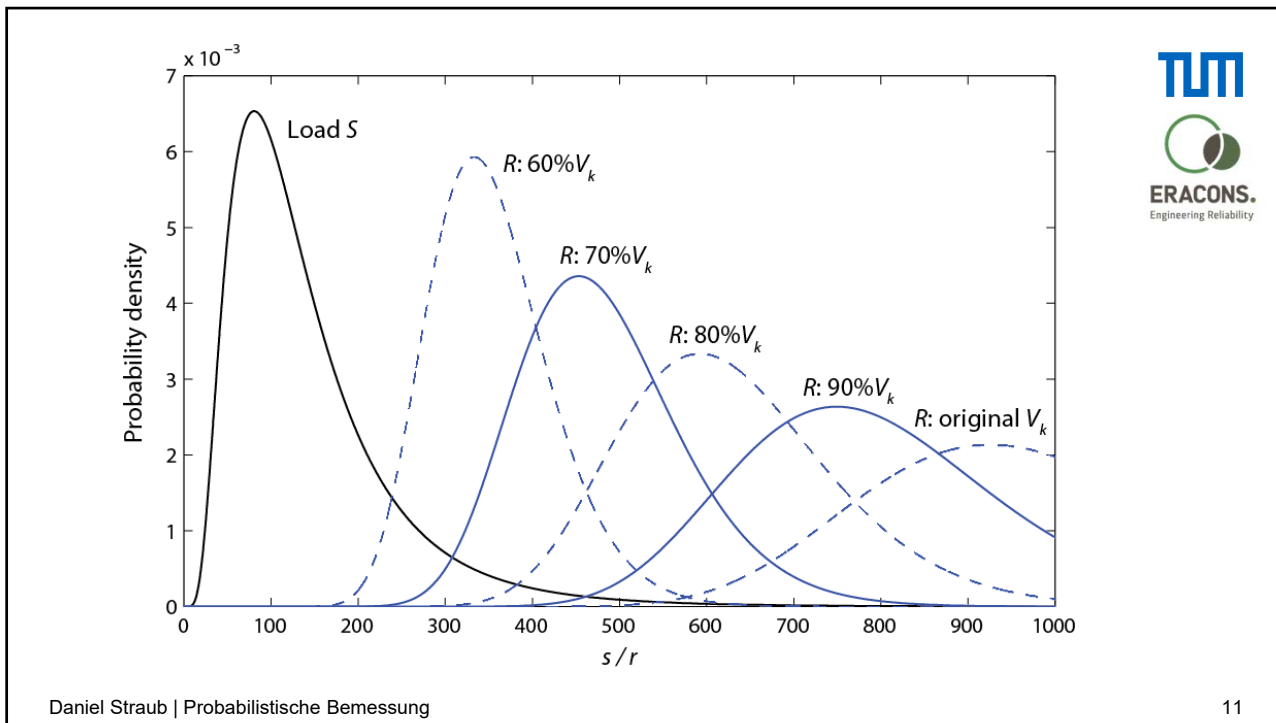
$$S = \alpha V_a^2 X_m$$

- $\alpha$ : constant (linear, thus irrelevant)
- $V_a$ : wind speed: Gumbel distributed
- $X_m$ : model uncertainty: lognormal distributed

- Resistance  $R$ : lognormal distributed

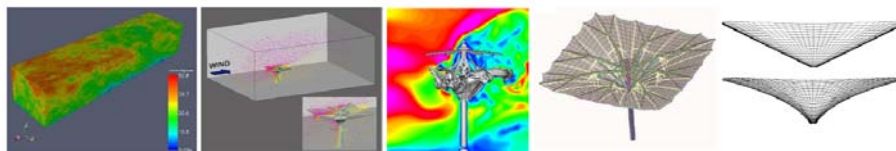
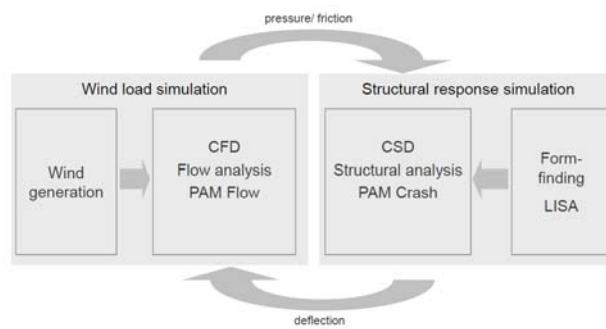
- The mean value of  $R$  can be determined from the partial safety factor (EC) design check





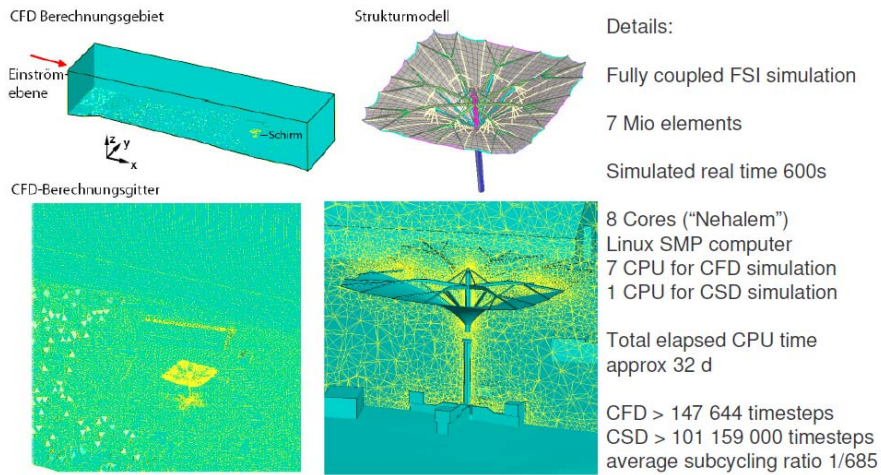
# Translating results from advanced numerical models into design values

## Computational Wind Simulation & Structural Analysis



Michalski, A., P. D. Kermel, et al. (2011). Validation of the computational fluid-structure interaction simulation at real-scale tests of a flexible 29 m umbrella in natural wind flow. *Journal of Wind Engineering and Industrial Aerodynamics* 99(4): 400-413.

## Computational efforts are significant Thus length of the simulation is limited



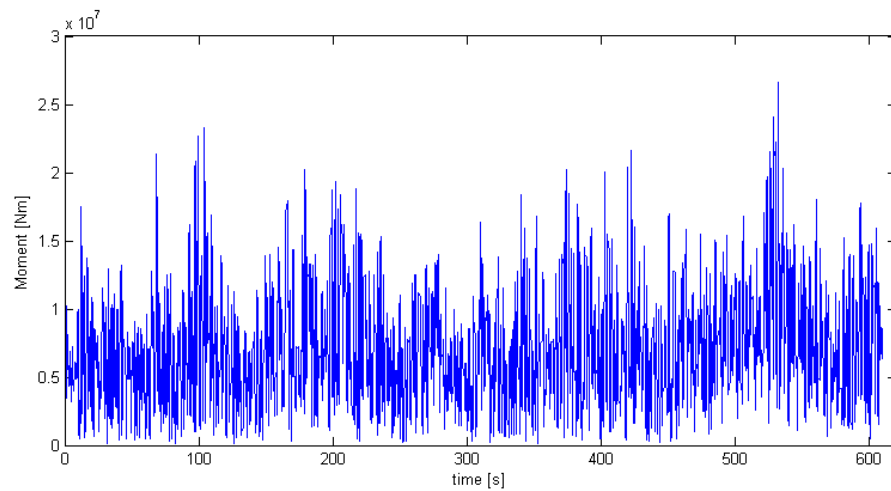
Michalski, A., P. D. Kermel, et al. (2011). Validation of the computational fluid-structure interaction simulation at real-scale tests of a flexible 29 m umbrella in natural wind flow. *Journal of Wind Engineering and Industrial Aerodynamics* 99(4): 400-413.

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## How to determine the design values?



Time series of a load effect resulting from characteristic (50 yr) wind:



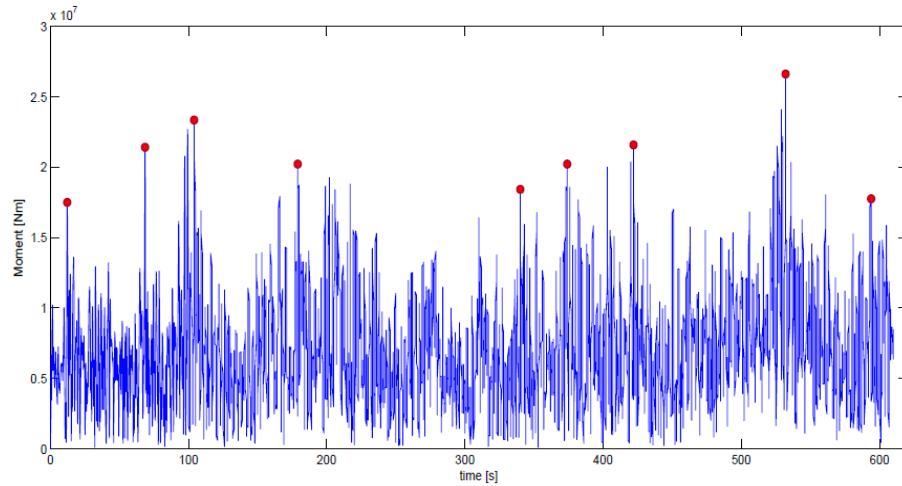
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## Step 1: Extreme value analysis

Peak-over-threshold:

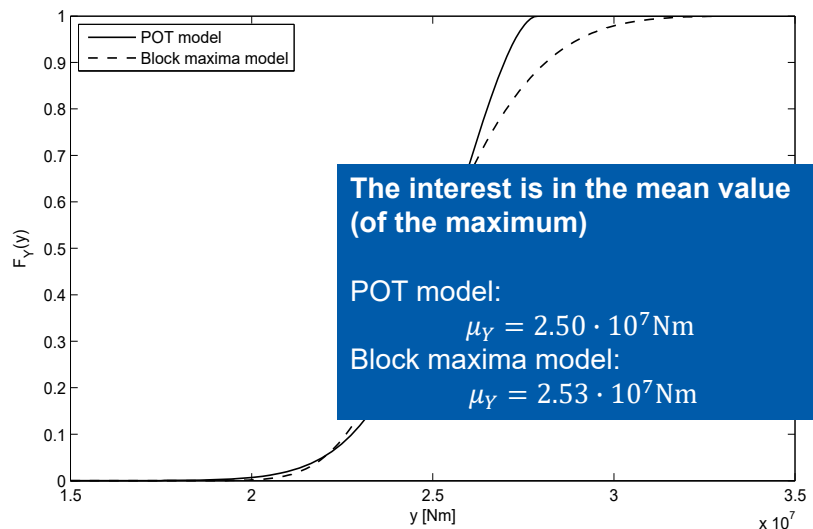
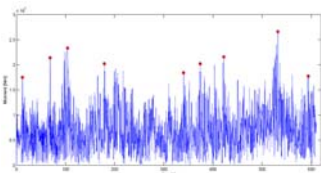


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## Step 1: Extreme value analysis

Peak-over-threshold to assess maximum in a 10min time window

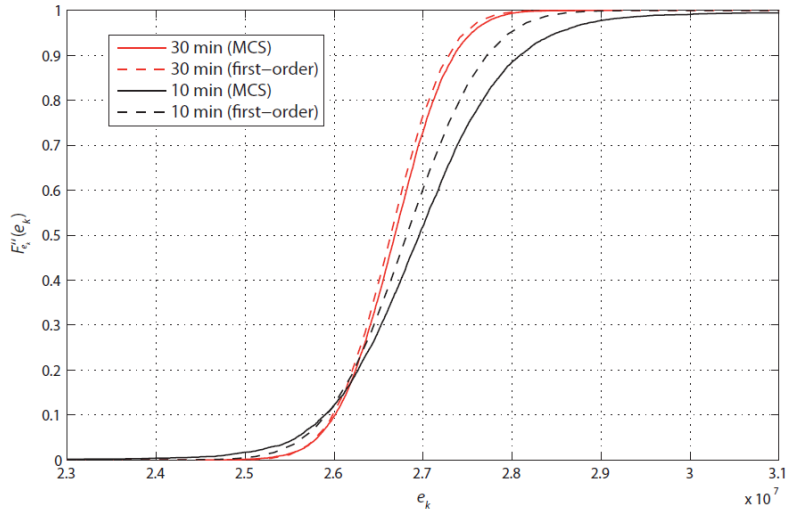


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## Step 2: Quantify statistical uncertainty

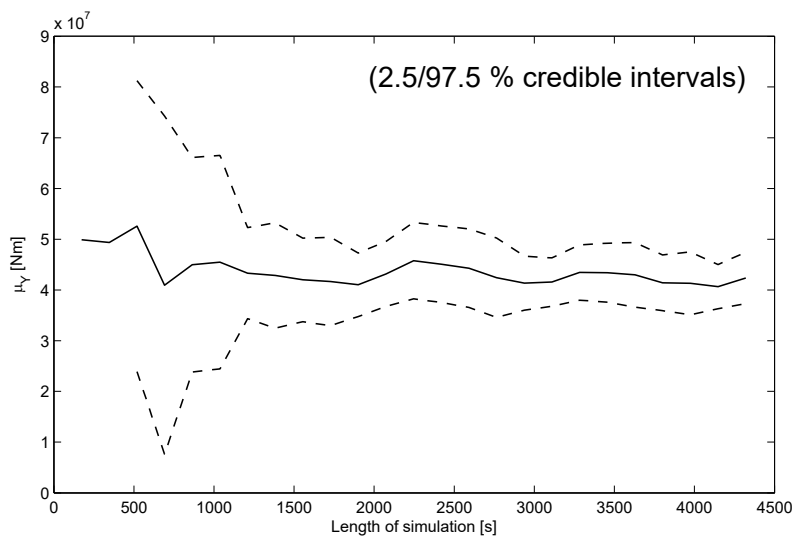


- Parameters  $\theta$  of EV models determined by a Bayesian approach
- Resulting statistical uncertainty in the load effect by propagating uncertainty in  $\theta$  to  $E_k(\theta)$



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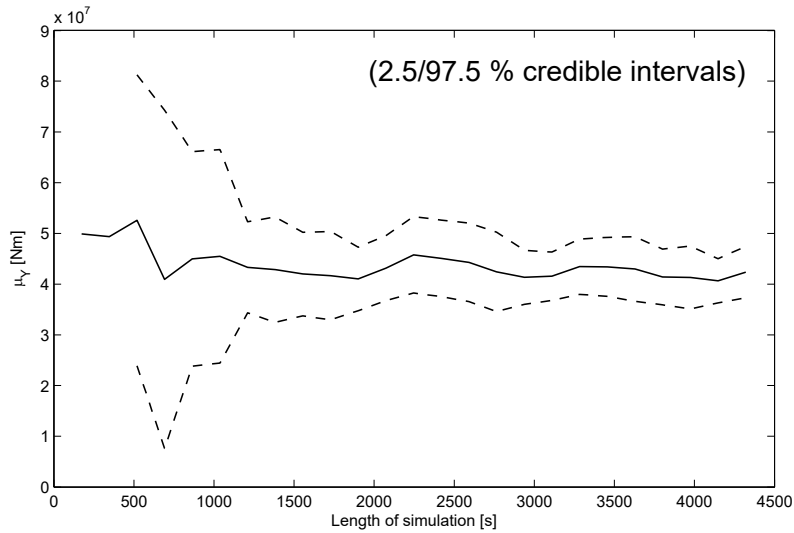
## Statistical uncertainty varies with length of simulation



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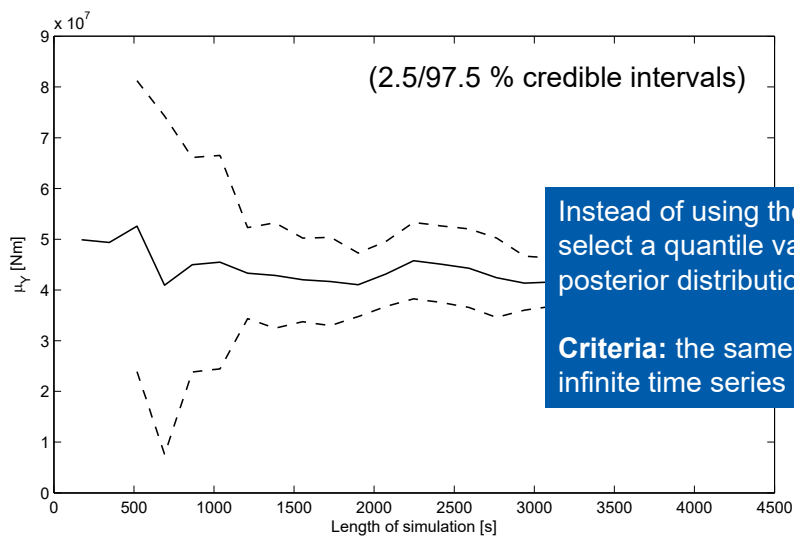
### Step 3: Compensate the statistical uncertainty



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### Step 3: Compensate the statistical uncertainty



Instead of using the mean value, select a quantile value in the posterior distribution of the mean

**Criteria:** the same reliability as if an infinite time series was available

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## Step 3: Compensate statistical uncertainty



Criteria: Reliability achieved with limited data should be equal to reliability with unlimited data

$$\beta_k^{(T,q)} = \beta_k^{(\infty)}$$

where:

$\beta_k$ : conditional reliability index (conditional on characteristic 50yr wind load scenario)

$T$ : duration of simulation

## Conditional reliability evaluation



Maximum load effect  $S$ :

$$S = Y \cdot Z_m$$

$Z_m$  : model error of the structural (FSI) analysis (LN distribution based on validation experiments)

$Y$  : maximum load, predictive distribution as obtained from data:

$$F_Y(y) = \int_{\theta} F_Y(y; \theta) f_{\theta}''(\theta) d\theta.$$

Limit state function:

$$g(R, Y, Z_m) = R - Y \cdot Z_m$$

## Conditional reliability evaluation

Distribution of  $R$  from:

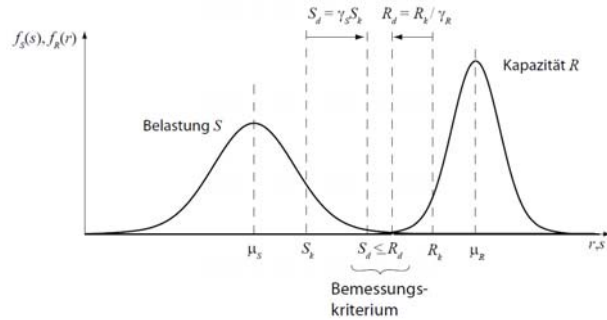
$$\frac{R_k}{\gamma_R} \geq E_{kq} \gamma_Q$$

thus:

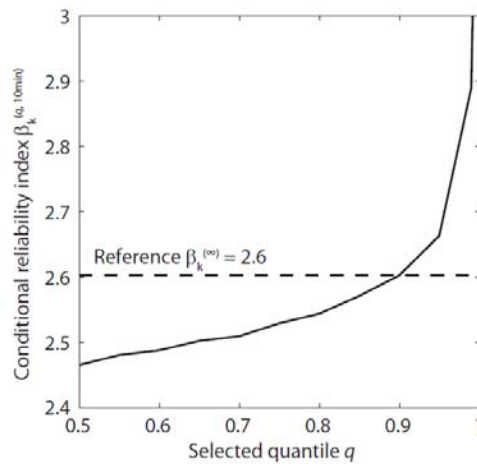
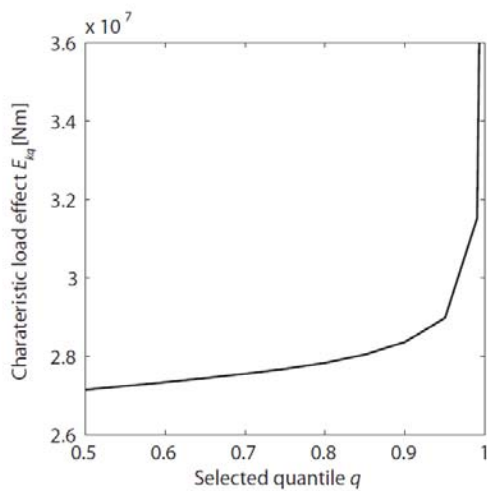
$$R_k = E_{kq} \gamma_Q \gamma_R$$

and

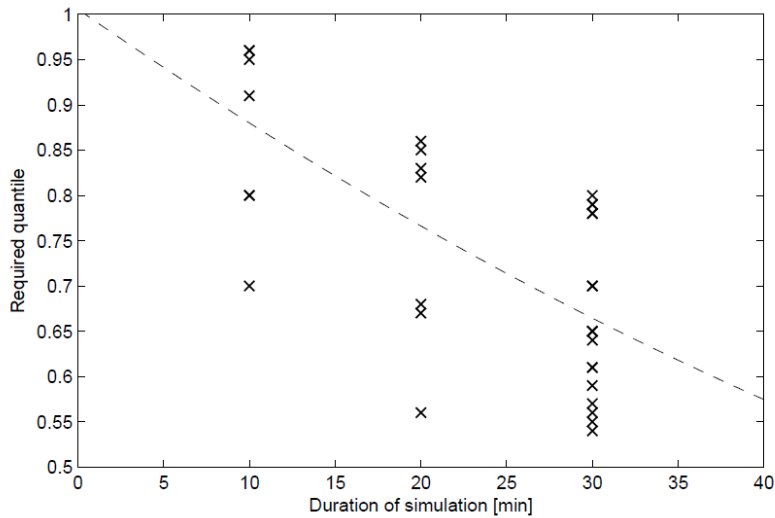
$$F_R^{-1}(0.05) = E_{kq} \gamma_Q \gamma_R$$



## Effect of quantile selection on characteristic load effect and on reliability



## Final result: required quantiles in function of time series length



Trade-off between computational cost (duration of simulation) and construction cost can be made

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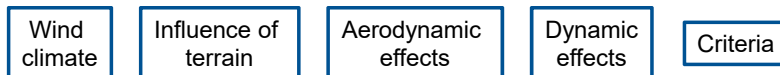
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Use of advanced design methods:  
The *hidden safety* challenge

## Eurocode: wind load model

Based on Davenport's wind loading chain (1987):



Wind load determined from

- Reference wind speed (98 [%] quantil of 10 min mean wind speeds at 10m)
- Gust-, Pressure-, Roughness-coefficient

## More precise predictions of wind loads are possible

This might be desirable from an economic point-of-view

E.g. replace code coefficients by a virtual wind tunnel

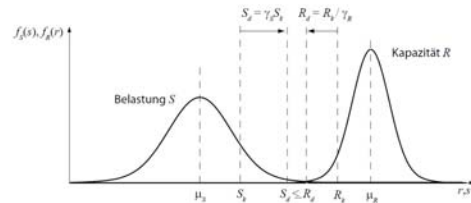
E.g. replace wind generic wind speed map by a site-specific, direction dependent wind speed prediction



## What is the effect of using advanced design methodologies?

Formal partial safety factor concept is based on:

- Characteristic values
- Partial safety factors



In reality, an additional safety element are conservative (parameter) choices in simplified models

This safety element can disappear if advanced models are utilized. What is the effect on the reliability?

## Numerical investigation

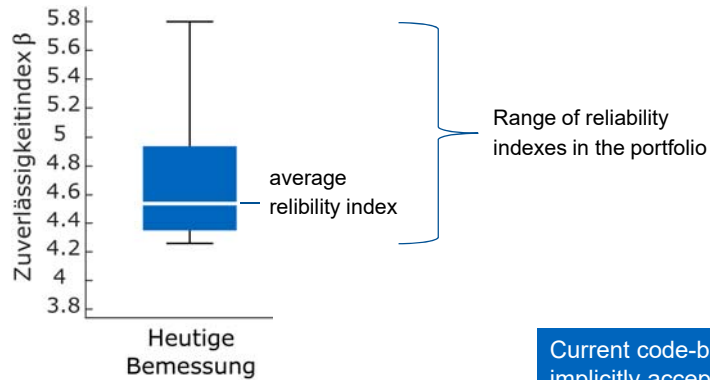
- Based on a representative portfolio of relevant structures (limit states) (as used in TC250 SC10 WG 1 for calibration of safety factors in Eurocode 0)
- Hidden safety is modeled through model uncertainties with biases
- Probabilistic model for hidden safeties follows Davenport (1987)

Confirmed with data from German weather service (DWD)

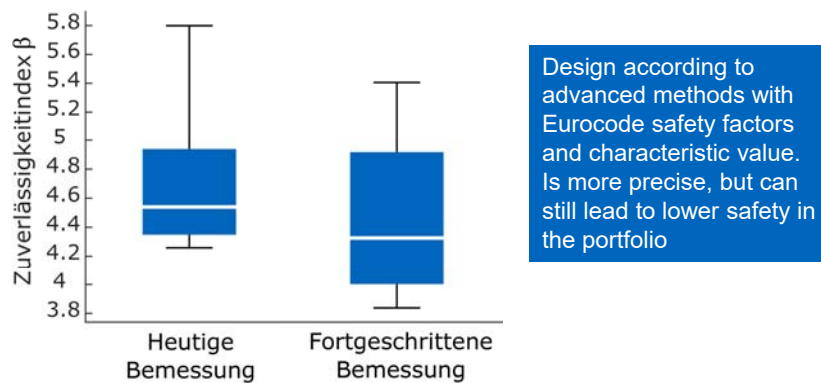
Quantity	Ratio: $\frac{\text{Expected}}{\text{Specified}}$	Coefficient of Variation
q (once-in-50 years)	0.8	0.2 - 0.3
$C_{exp}$	0.8	0.1 - 0.2
$C_{shp}$	0.9	0.1 - 0.2
$C_{dyn}$	1.0	0.1 - 0.2
$\zeta_{str}$ (damping)	1.0	0.4 - 0.5



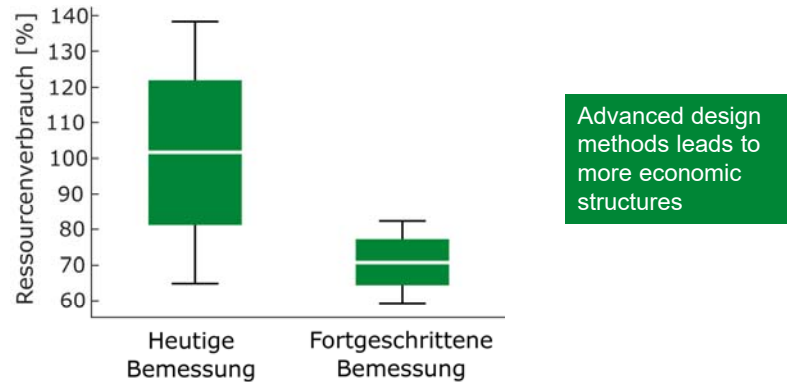
## Numerical investigation



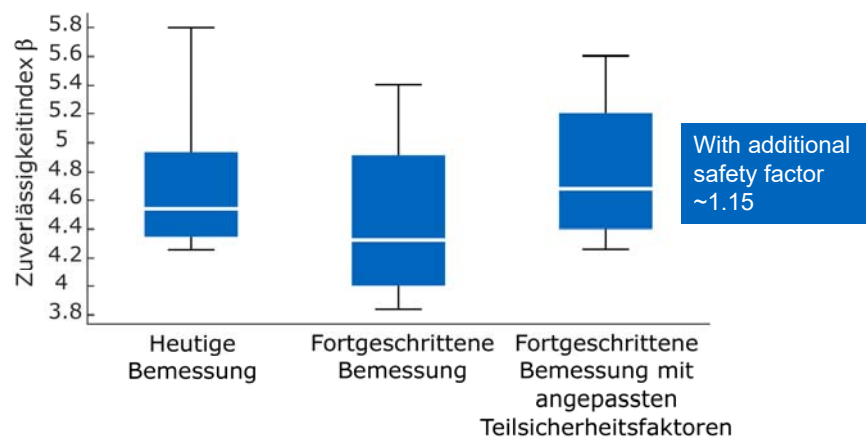
## Numerical investigation



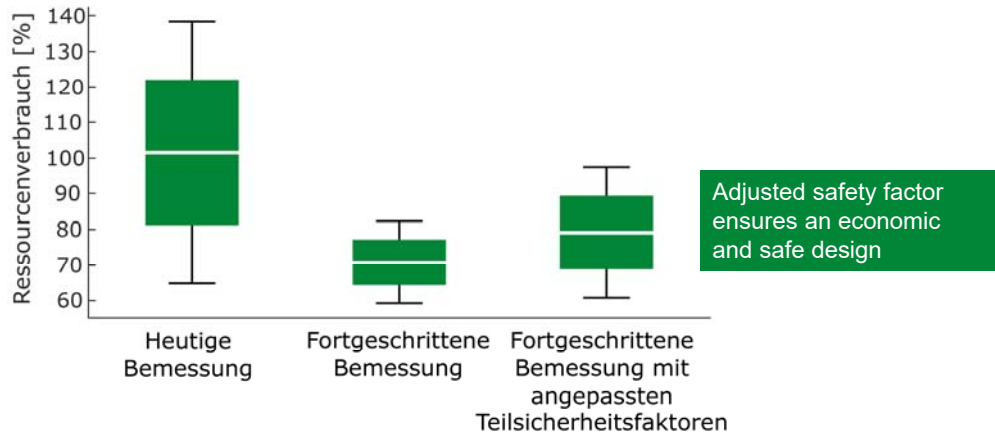
## Numerical investigation: economic aspects



## Additional safety factor to compensate for lost hidden safety



## Additional safety factor to compensate for lost hidden safety

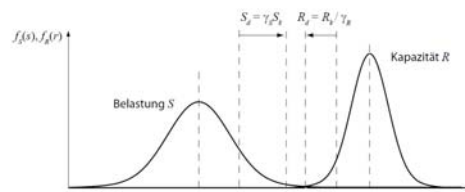


## What is the effect of using advanced design methodologies?

Formal partial safety factor concept is based on:

- Characteristic values
- Partial safety factors

In reality, an additional safety element are conservative (parameter) choices in simplified models



We do not really know how conservative they are (but often reflect legacy experience)

This safety element can disappear if advanced models are utilized. What is the effect on the reliability?

## Conclusions



- Considerations based on simple probabilistic models enable a rational approach to identifying characteristic values and safety factors consistent with code requirements
- They can also address the uncertainty associated with limited knowledge and data
- When replacing Eurocode wind load design equations with advanced models, hidden safety should be addressed (possibly with additional safety factors) to maintain the reliability level





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